Foundation Engineering, 2018-2019

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SITE INVESTIGATION

**Syllabus of Foundation Design**

Site Investigation, Bearing capacity of Soil, Settlement, Foundation Design, Piles, Lateral Earth Pressure, slope Stability.
SITE INVESTIGATION
The process of determining the layers of natural soil deposits that will underlie a proposed structure and their physical properties is generally referred to as site investigation.
The purpose of a soil investigation program

1. Selection of the type and the depth of foundation suitable for a given structure.
2. Evaluation of the load-bearing capacity of the foundation.
3. Estimation of the probable settlement of a structure.
4. Determination of potential foundation problems (for example, expansive soil, collapsible soil, sanitary landfill, and so on).
5. Establishment of ground water table.
6. Prediction of lateral earth pressure for structures like retaining walls, sheet pile bulkheads, and braced cuts.
The purpose of the exploration program is to determine, within practical limits, the stratification and engineering properties of the soils underlying the site. The principal properties of interest will be the strength, deformation, and hydraulic characteristics. The program should be planned so that the maximum amount of information can be obtained at minimum cost.
Steps of subsurface exploration program

[Stage 1]

1. Assembly of all available information on dimensions, column spacing, type and use of the structure, basement requirements, and any special architectural considerations of the proposed building. Foundation regulations in the local building code should be consulted for any special requirements. For bridges the soil engineer should have access to type and span lengths as well as pier loadings. This information will indicate any settlement limitations, and can be used to estimate foundation loads.
Steps of subsurface exploration program

[Stage 2]

2. Reconnaissance of the area:

This may be in the form of a field trip to the site which can reveal information on the type and behavior of adjacent structures such as cracks, noticeable sags, and possibly sticking doors and windows. The type of local existing structure may influence, to a considerable extent, the exploration program and the best foundation type for the proposed adjacent structure.
3. A preliminary site investigation:

In this phase a few borings are made or a test pit is opened to establish in a general manner the stratification, types of soil to be expected, and possibly the location of the groundwater table. One or more borings should be taken to rock, or competent strata, if the initial borings indicate the upper soil is loose or highly compressible. This amount of exploration is usually the extent of the site investigation for small structures.
Steps of subsurface exploration program

[Stage 4]

4. A detailed site investigation:

Where the preliminary site investigation has established the feasibility of the project, a more detailed exploration program is undertaken. The preliminary borings and data are used as a basis for locating additional borings, which should be confirmatory in nature, and determining the additional samples required.
Depth of Boring

The approximate required minimum depth of the borings should be predetermined. The estimated depths can be changed during the drilling operation, depending on the subsoil encountered. To determine the approximate minimum depth of boring, engineers may use the following rule:
1. Determine the net increase of stress, $\Delta \sigma$ under a foundation with depth as shown in the Figure.
2. Estimate the variation of the vertical effective stress, $\sigma'_v$, with depth.
3. Determine the depth, $D = D_1$, at which the stress increase $\Delta \sigma$ is equal to $(1/10)$ $q$ ($q =$ estimated net stress on the foundation).
4. Determine the depth, $D = D_2$, at which $\Delta \sigma / \sigma'_v = 0.05$.
5. Unless bedrock is encountered, the smaller of the two depths, $D_1$ and $D_2$, just determined is the approximate minimum depth of boring required. Table shows the minimum depths of borings for buildings based on the preceding rule.
Depth of Boring

Determination of the minimum depth of boring

Effective overburden pressure

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## Depth of Boring

### Depth of Boring

<table>
<thead>
<tr>
<th>BUILDING WIDTH (m)</th>
<th>Number of Stories (S)</th>
<th>Boring Depth (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>1</td>
<td>2</td>
</tr>
<tr>
<td>30.5</td>
<td>3.4</td>
<td>6.1</td>
</tr>
<tr>
<td>61.0</td>
<td>3.7</td>
<td>6.7</td>
</tr>
<tr>
<td>122.0</td>
<td>3.7</td>
<td>7.0</td>
</tr>
</tbody>
</table>

Example: A 4-story building and its width = 61m, so the minimum depth of boring = 12.5 m

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Depth of Boring

For hospitals and office buildings, the following rule could be used to determine boring depth

\[ D_b = 3S^{0.7} \] (for light steel or narrow concrete buildings)

\[ D_b = 6S^{0.7} \] (for heavy steel or wide concrete buildings)

where:
\( D_b = \) depth of boring, in meters
\( S = \) number of stories
Depth of Boring

When deep excavations are anticipated, the depth of boring should be at least 1.5 times the depth of excavation. Sometimes subsoil conditions are such that the foundation load may have to be transmitted to the bedrock. The minimum depth of core boring into the bedrock is about 3m. If the bedrock is irregular or weathered, the core borings may have to be extended to greater depths.
Spacing of Boring

There are no hard and fast rules for the spacing of the boreholes. The following table gives some general guidelines for borehole spacing. These spacing can be increased or decreased, depending on the subsoil condition. If various soil strata are more or less uniform and predictable, the number of boreholes can be reduced.
## Approximate Spacing of Boreholes

<table>
<thead>
<tr>
<th>Type of Project</th>
<th>Spacing (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Multistory Building</td>
<td>10-30</td>
</tr>
<tr>
<td>One story Industrial Plants</td>
<td>20-60</td>
</tr>
<tr>
<td>Highways</td>
<td>250-500</td>
</tr>
<tr>
<td>Residential Subdivision</td>
<td>250-500</td>
</tr>
<tr>
<td>Dams and Dikes</td>
<td>40-80</td>
</tr>
</tbody>
</table>
Example:

For a 4–story residential building of plan area 270 m\(^2\) with pressure is equal to 150 kPa at the level of the foundation base (q) that transferred from the superstructure. The expected unit weight of the soil under the foundation is 15.5 kN/m\(^3\). Calculate the approximate depth of boring and the required boreholes number for reporting the soil investigation program thoroughly.
The earliest method of obtaining a test hole was to excavate a test pit using a pick and shovel. Because of economics, the current procedure is to use power-excavation equipment such as a backhoe to excavate the pit and then to use hand tools to remove a block sample or shape the site for in situ testing. This is the best method at present for obtaining quality undisturbed samples or samples for testing at other than vertical orientation.
SOIL BORING

- **Trial Pit**
  - 1-2 m width
  - 2-4 m depth

- **Bore hole**
  - 75 mm dia
  - 10-30 m depth

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Boring tools

Hand tools
- Posthole auger
- Helical auger

Auger boring

Power drills
Boring tools

Wash boring rig

- Single or double crown sheave-hook for multiple blocks for pulling of casing
- Three of four-legged derrick standard pipe or timber
- Swivel
- T-section or water swivel
- Water hose
- Drill rod
- T-section for return flow
- Motor
- Nipple
- Cat-head (Nigger-head)
- Coupling
- Pump
- Sump for wash water and collection of wash samples
- Drill rod coupling
- Casing coupling
- Casing
- Drive shoe
- Drill bit

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Rotary Drilling rig
Boring tools

Sampling devices: (a) scraper bucket

Drill rod

Driving point

Section at $S - S$

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Preparation of Boring Logs

1. Name and address of the drilling company
2. Driller’s name
3. Job description and number
4. Number, type, and location of boring
5. Date of boring
6. Subsurface stratification, which can be obtained by visual observation of the soil brought out by auger, split-spoon sampler, and thin-walled Shelby tube sampler
7. Elevation of water table and date observed, use of casing and mud losses, and so on
8. Standard penetration resistance and the depth of SPT
9. Number, type, and depth of soil sample collected
10. In case of rock coring, type of core barrel used and, for each run, the actual length of coring, length of core recovery, and ROD

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### Boring Logs

#### Example Sheet

<table>
<thead>
<tr>
<th>Depth (ft)</th>
<th>Symbol</th>
<th>Description</th>
<th>Number</th>
<th>Type</th>
<th>Recovery</th>
<th>Vapour</th>
</tr>
</thead>
<tbody>
<tr>
<td>0</td>
<td>Ground Surface</td>
<td></td>
<td>0</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>92</td>
<td>Asphal</td>
<td>Sand and gravel fill, some organic debris.</td>
<td>1</td>
<td>40</td>
<td>180</td>
<td></td>
</tr>
<tr>
<td>93</td>
<td>Fill</td>
<td>Sand and gravel fill, some organic debris.</td>
<td>2</td>
<td>30</td>
<td>220</td>
<td></td>
</tr>
<tr>
<td>94</td>
<td>Sandy Silt</td>
<td>Moist, brown to grey sandy silt with embedded gravel.</td>
<td>3</td>
<td>75</td>
<td>380</td>
<td></td>
</tr>
<tr>
<td>95</td>
<td>Sand</td>
<td>Medium to fine sand, occasional clay lenses. Strong hydrocarbon odour.</td>
<td>4</td>
<td>60</td>
<td>450</td>
<td></td>
</tr>
<tr>
<td>96</td>
<td>Clay</td>
<td>Mottled brown and grey silty clay. Some sandy lenses.</td>
<td>5</td>
<td>60</td>
<td>315</td>
<td></td>
</tr>
<tr>
<td>97</td>
<td>Sand</td>
<td>Compact, coarse to medium sand. Shell fragments.</td>
<td>6</td>
<td>45</td>
<td>125</td>
<td></td>
</tr>
<tr>
<td>98</td>
<td></td>
<td></td>
<td>7</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>99</td>
<td></td>
<td></td>
<td>8</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>100</td>
<td></td>
<td></td>
<td>9</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>101</td>
<td></td>
<td></td>
<td>10</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

**Drilled By:** ABC Drilling Company

**Drill Method:** HVS Auger

**Drill Date:** 02-06-2000

**Hole Size:** 12" 
**Datum:** Local 
**Sheet:** 1 of 1
<table>
<thead>
<tr>
<th>Depth (m)</th>
<th>Soil Description</th>
<th>Sample No.</th>
<th>Type</th>
<th>Depth (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td>0</td>
<td>Topsoil, sandy (0-0.3 m)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>0.3</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>0.5</td>
<td>Loose brown moist fine sand (0.3-1.2 m)</td>
<td>1</td>
<td>Auger</td>
<td>0.8</td>
</tr>
<tr>
<td>1.0</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1.2</td>
<td>Dense silty sand 12-1.8 m</td>
<td>2</td>
<td>Split spoon</td>
<td>1.2-1.6</td>
</tr>
<tr>
<td>1.5</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1.8</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>2.0</td>
<td>Till, clayey, with some silt, moist,</td>
<td>3</td>
<td>Split spoon</td>
<td>2.1-2.5</td>
</tr>
<tr>
<td>2.5</td>
<td>hard (1.8-2.8 m)</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>2.8</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>3.0</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>3.5</td>
<td>Clay, soft wet (2.8-3.6 m)</td>
<td>4</td>
<td>Shelby</td>
<td>3.0-3.6</td>
</tr>
<tr>
<td>3.6</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

End of test hole

**Figure 2-7** Typical field notes.
Figure 2-10 Test-hole log.
### Boring Log

**Name of the Project**: Two-story apartment building  
**Location**: Johnson & Olive St.  
**Date of Boring**: March 2, 1982  
**Boring No.**: 3  
**Type of Boring**: Hollow stem auger  
**Ground Elevation**: 60.8 m

<table>
<thead>
<tr>
<th>Soil Description</th>
<th>Depth (m)</th>
<th>Soil Sample Type and Number</th>
<th>N</th>
<th>$w_n$ (%)</th>
<th>Comments</th>
</tr>
</thead>
<tbody>
<tr>
<td>Light brown clay (fill)</td>
<td>1</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Silty sand (SM)</td>
<td>2</td>
<td>SS-1</td>
<td>9</td>
<td>8.2</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3</td>
<td>SS-2</td>
<td>12</td>
<td>17.6</td>
<td>$LL = 38$</td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td>$PI = 11$</td>
</tr>
<tr>
<td><em>G.W.T. 3.5 m</em></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Light gray silty clay (ML)</td>
<td>4</td>
<td>ST-1</td>
<td>20.4</td>
<td>36</td>
<td>$LL = 36$</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td></td>
<td></td>
<td></td>
<td>$q_u = 112 \text{kN/m}^2$</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>SS-3</td>
<td>11</td>
<td>20.6</td>
<td></td>
</tr>
<tr>
<td>Sand with some gravel (SP)</td>
<td>7</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>End of boring @ 8 m</td>
<td>8</td>
<td>SS-4</td>
<td>27</td>
<td>9</td>
<td></td>
</tr>
</tbody>
</table>

- $N = \text{standard penetration number (below/304.8 mm)}$  
- $w_n = \text{natural moisture content}$  
- $LL = \text{liquid limit}$; $PI = \text{plasticity index}$  
- $q_u = \text{unconfined compression strength}$  
- SS = split-spoon sample; ST = Shelby tube sample  
- *Ground water table observed after one week of drilling*
Soils Investigation

<table>
<thead>
<tr>
<th>Gravel</th>
<th>Sand</th>
<th>Silt</th>
<th>Clay</th>
<th>Organic</th>
<th>Till</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sandy top soil</td>
<td>Gravelly sand</td>
<td>Silty sand</td>
<td>Sandy silt</td>
<td>Silty clay</td>
<td>Sandy till</td>
</tr>
</tbody>
</table>

Water Table

Bed Rock

Type of Sample

S.S. – Split Spoon
S.T. – Shelby Tube
A.S. – Auger Sample
W.S. – Washed Sample
R.C. – Rock Core
V.S. – Vane Shear Sample

Soil Tests

F.V. – Field Vane
L.V. – Lab Vane
Qu – Unconfined Compression
Qq – Undrained (quick) Triaxial
C – Dynamic Cone (Blows per foot)

Typical test-hole symbols and abbreviations.
Two types of soil samples can be obtained during sampling disturbed and undisturbed. The most important engineering properties required for foundation design are strength, compressibility, and permeability. Reasonably good estimates of these properties for cohesive soils can be made by laboratory tests on undisturbed samples which can be obtained with moderate difficulty. It is nearly impossible to obtain a truly undisturbed sample of soil; so in general usage the term "undisturbed" means a sample where some precautions have been taken to minimize disturbance or remolding effects. In this context, the quality of an "undisturbed" sample varies widely between soil laboratories.
Soils Investigation

Table 2-4
LABORATORY TESTS RELATED TO A SOILS INVESTIGATION

<table>
<thead>
<tr>
<th>Test</th>
<th>Sample Required</th>
<th>Soils</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Disturbed or Undisturbed</td>
<td>Undisturbed</td>
</tr>
<tr>
<td>Moisture content</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>Grain size</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>Atterberg Limits</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>Relative density (specific gravity)</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>Density (unit weight)</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>Unconfined compression</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>Triaxial compression</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>Direct shear</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>Consolidation</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>Vane shear</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>Permeability</td>
<td>X</td>
<td></td>
</tr>
</tbody>
</table>

2-3.2 Approximate values for soil strength may be obtained from simple field tests, as indicated in Table 2-5.
Disturbed vs Undisturbed

- Good quality samples necessary.

\[ A_R < 10\% \]

- Thicker the wall, greater the disturbance.

\[ A_R = \frac{O.D.^2 - I.D.^2}{I.D.^2} \times 100(\%) \]

Good quality samples necessary.

Thicker the wall, greater the disturbance.
Disturbed vs Undisturbed

- samples (disturbed) collected in split-spoon sampler

$A_R = 112\%$; use for classification

I.D. = 35 mm
O.D. = 51 mm
# Common Sampling Methods

<table>
<thead>
<tr>
<th>Sampler</th>
<th>Disturbed / Undisturbed</th>
<th>Appropriate Soil Types</th>
<th>Method of Penetration</th>
<th>% Use in Practice</th>
</tr>
</thead>
<tbody>
<tr>
<td>Split-Barrel (Split Spoon)</td>
<td>Disturbed</td>
<td>Sands, silts, clays</td>
<td>Hammer driven</td>
<td>85</td>
</tr>
<tr>
<td>Thin-Walled Shelby Tube</td>
<td>Undisturbed</td>
<td>Clays, silts, fine-grained soils, clayey sands</td>
<td>Mechanically Pushed</td>
<td>6</td>
</tr>
<tr>
<td>Continuous Push</td>
<td>Partially Undisturbed</td>
<td>Sands, silts, &amp; clays</td>
<td>Hydraulic push with plastic lining</td>
<td>4</td>
</tr>
<tr>
<td>Piston</td>
<td>Undisturbed</td>
<td>Silts and clays</td>
<td>Hydraulic Push</td>
<td>1</td>
</tr>
<tr>
<td>Pitcher</td>
<td>Undisturbed</td>
<td>Stiff to hard clay, silt, sand, partially weather rock, and frozen or resin impregnated granular soil</td>
<td>Rotation and hydraulic pressure</td>
<td>&lt;1</td>
</tr>
<tr>
<td>Denison</td>
<td>Undisturbed</td>
<td>Stiff to hard clay, silt, sand and partially weather rock</td>
<td>Rotation and hydraulic pressure</td>
<td>&lt;1</td>
</tr>
<tr>
<td>Modified California</td>
<td>Disturbed</td>
<td>Sands, silts, clays, and gravels</td>
<td>Hammer driven (large split spoon)</td>
<td>&lt;1</td>
</tr>
<tr>
<td>Continuous Auger</td>
<td>Disturbed</td>
<td>Cohesive soils</td>
<td>Drilling w/ Hollow Stem Augers</td>
<td>&lt;1</td>
</tr>
<tr>
<td>Bulk</td>
<td>Disturbed</td>
<td>Gravels, Sands, Silts, Clays</td>
<td>Hand tools, bucket augering</td>
<td>&lt;1</td>
</tr>
<tr>
<td>Block</td>
<td>Undisturbed</td>
<td>Cohesive soils and frozen or resin impregnated granular soil</td>
<td>Hand tools</td>
<td>&lt;1</td>
</tr>
</tbody>
</table>
ROCK SAMPLING

• Rock cores are necessary if the soundness of the rock is to be established.
• Small cores tend to break up inside the drill barrel.
• Larger cores also have a tendency to break up (rotate inside the barrel and degrade), especially if the rock is soft or fissured.

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Rock coring

Rock coring: (a) single-tube core barrel; (b) double-tube core barrel

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**ROCK SAMPLING - Definition**

**Recovery Ratio**

\[
\text{Recovery Ratio} = \frac{\sum \text{Lengths of intact pieces of core}}{\text{Length of core advance}}
\]

**RQD**

\[
\text{RQD} = \frac{\sum \text{Lengths of intact pieces of core } \geq 10.16 \text{ cm}}{\text{Length of core advance}}
\]
Rock Core Drilling

• Done with either tungsten carbide or diamond core bits
• Use a double or triple tube core barrel when sampling weathered or fractured rock
• Used to determine Rock Quality Designation

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Rock Quality Designation (RQD)

<table>
<thead>
<tr>
<th>CR</th>
<th>LENGTH (M)</th>
<th>TCR (%)</th>
<th>SCR (%)</th>
<th>ROD (%)</th>
<th>DATE</th>
</tr>
</thead>
<tbody>
<tr>
<td>90</td>
<td>189.90-191.90</td>
<td>100</td>
<td>100</td>
<td>97</td>
<td>07-04-04</td>
</tr>
<tr>
<td>91</td>
<td>191.90-194.10</td>
<td>98</td>
<td>98</td>
<td>98</td>
<td>07-04-04</td>
</tr>
<tr>
<td>92</td>
<td>194.10-195.90</td>
<td>97</td>
<td>97</td>
<td>94</td>
<td>07-04-04</td>
</tr>
<tr>
<td>93</td>
<td>195.90-197.05</td>
<td>98</td>
<td>98</td>
<td>98</td>
<td>07-04-04</td>
</tr>
</tbody>
</table>

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Rock Quality Designation

RQD
Rock Quality Designation (RQD) is defined as the percentage of rock cores that have length equal or greater than 10 cm over the total drill length.

\[ \text{RQD} = \frac{\sum \text{Li}}{\text{L}} \times 100\%, \quad \text{Li} > 10 \text{ cm} \]

\[ \text{RQD} = \frac{(L1 + L2 + \ldots + Ln)}{L} \times 100\% \]

<table>
<thead>
<tr>
<th>RQD</th>
<th>Rock Mass Quality</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt; 25</td>
<td>Very poor</td>
</tr>
<tr>
<td>25 – 50</td>
<td>Poor</td>
</tr>
<tr>
<td>50 – 75</td>
<td>Fair</td>
</tr>
<tr>
<td>75 – 90</td>
<td>Good</td>
</tr>
<tr>
<td>99 – 100</td>
<td>Excellent</td>
</tr>
</tbody>
</table>

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Example on Core Recovery & RQD

- Core run of 150 cm
- Total core recovery = 125 cm
- Core recovery ratio = 125/150 = 83%
- On modified basis, 95 cm are counted

RQD = 95/150=63 %

<table>
<thead>
<tr>
<th>Core Recovery cm</th>
<th>Modified Core Recovery, cm</th>
</tr>
</thead>
<tbody>
<tr>
<td>25</td>
<td>25</td>
</tr>
<tr>
<td>5</td>
<td>0</td>
</tr>
<tr>
<td>5</td>
<td>0</td>
</tr>
<tr>
<td>7.5</td>
<td>0</td>
</tr>
<tr>
<td>10</td>
<td>10</td>
</tr>
<tr>
<td>12.5</td>
<td>12.5</td>
</tr>
<tr>
<td>7.5</td>
<td>0</td>
</tr>
<tr>
<td>10</td>
<td>10</td>
</tr>
<tr>
<td>15</td>
<td>15</td>
</tr>
<tr>
<td>10</td>
<td>10</td>
</tr>
<tr>
<td>5</td>
<td>0</td>
</tr>
<tr>
<td>12.5</td>
<td>12.5</td>
</tr>
<tr>
<td>125</td>
<td>95</td>
</tr>
</tbody>
</table>
Groundwater conditions and the potential for groundwater seepage are fundamental factors in virtually all geotechnical analyses and design studies. Accordingly, the evaluation of groundwater conditions is a basic element of almost all geotechnical investigation programs. Groundwater investigations are of two types as follows:

• Determination of groundwater levels and pressures.
• Measurement of the permeability of the subsurface materials.
FIELD STRENGTH TESTS

The following are the major field tests for determining the soil strength:

1. Vane shear test (VST).
2. Standard Penetration Test (SPT).
3. Cone Penetration Test (CPT).
4. The Borehole Shear Test (BST).
5. The Flat Dilatometer Test (DMT).
6. The Pressure-meter Test (PMT).
7. The Plate Load Test (PLT).
FIELD STRENGTH TESTS

SPT

VST

PMT

CPT

DMT

In bore holes
Standard Penetration Test (SPT)

SPT Resistance (N-value) is total number of blows to drive sampler the 2nd and 3rd 6” increments

140 lb Hammer dropping Anvil 30”

Anvil

Drill Rod

Split-Barrel Drive sampler

Seating Spoon = 6”

Second Increment = 6”

Third Increment = 6”

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Standard Penetration Test (SPT)
Standard Penetration Test (SPT)

Corrections are normally applied to the SPT blow count to account for differences in:

- **energy imparted** during the test (60% hammer efficiency)
- **the stress level** at the test depth

The following equation is used to compensate for the testing factors (Skempton, 1986):
Standard Penetration Test (SPT)

\[ N_{60} = 1.67 \times E_m \times C_b \times C_r \times N \]

where \( N_{60} \) = SPT \( N \)-value corrected for field testing procedures
\( E_m \) = hammer efficiency (for U.S. equipment, \( E_m \) equals 0.6 for a safety hammer and equals 0.45 for a doughnut hammer)
\( C_b \) = borehole diameter correction (\( C_b = 1.0 \) for boreholes of 65- to 115-mm diameter, 1.05 for 150-mm diameter, and 1.15 for 200-mm diameter hole)
\( C_r \) = rod length correction (\( C_r = 0.75 \) for up to 4 m of drill rods, 0.85 for 4 to 6 m of drill rods, 0.95 for 6 to 10 m of drill rods, and 1.00 for drill rods in excess of 10 m)
\( N \) = measured SPT \( N \)-value
Standard Penetration Test (SPT)

Empirical correlation between SPT $N_{60}$ value, vertical effective stress, and friction angle for clean quartz sand deposits. (Adapted from DeMello, 1971; reproduced from Coduto, 1994.)
Cone Penetration Test (CPT)

friction ratio, $f_R = \frac{f_s}{q_c} \times 100\%$

sleeve friction ($f_s$)

cone resistance ($q_c$) or tip resistance ($q_t$)

Typically 0 —— 10%.

granular —— cohesive
Cone Penetration Test (CPT)
Cone Penetration Test (CPT)
Cone Penetration Test (CPT)

**SCPT Correlations**

*In Clays,*

\[ c_u = \frac{q_c - \sigma_{vo}}{N_k} \]

cone factor (15-20); varies with cone

*In Sands,*

\[ E = 2.5-3.5 \ q_c \] (for young normally consolidated sands)
Cone Penetration Test (CPT)

$\phi'$ from SPT/CPT in Granular Soils

After Peck et al. (1974)  
After Meyerhof (1976)
The Plate Load Test (PLT)
The Plate Load Test (PLT)

Dead weight of a truck or a beam attached to anchor piles

Props for stability when using dead weights.

Jack

Short block

Steel plate

anchors or piles to provide a reactive force

Several dial gauges attached to an independent suspension system to record plate settlements with each increment of the jack load.
The Plate Load Test (PLT)
The Plate Load Test (PLT)
Scale Effect in Foundation Design
Geotechnical Design Reports

• At the end of all subsoil exploration programs, the soil and/or rock specimens collected from the field are subjected to visual observation and appropriate laboratory testing. After the compilation of all of the required information, a soil exploration report is prepared for the use of the design office and for reference during future construction work. Although the details and sequence of information in the report may vary to some degree is depending on the structure under consideration and the person compiling the report.
Subsoil Exploration Report

1. A description of the scope of the investigation
2. A description of the proposed structure for which the subsoil exploration has been conducted
3. A description of the location of the site, including any structures nearby, drainage conditions, the nature of vegetation on the site and surrounding it, and any other features unique to the site
4. A description of the geological setting of the site
5. Details of the field exploration—that is, number of borings, depths of borings, types of borings involved, and so on
6. A general description of the subsoil conditions, as determined from soil specimens and from related laboratory tests, standard penetration resistance and cone penetration resistance, and soon
7. A description of the water-table conditions
8. Recommendations regarding the foundation, including the type of foundation recommended, the allowable hearing pressure, and any special construction procedure that may be needed; alternative foundation design procedures should also be discussed in this portion of the report
9. Conclusions and limitations of the investigations
The following graphical presentations should be attached to the report:

1. A site location map
2. A plan view of the location of the borings with respect to the proposed structures and those nearby
3. Boring logs
4. Laboratory test results
5. Other special graphical presentations
Example: Table of Contents for a Geotechnical Investigation (Data) Report

1.0 INTRODUCTION
2.0 SCOPE OF WORK
3.0 SITE DESCRIPTION
4.0 FIELD INVESTIGATION PROGRAM & IN-SITU TESTING
5.0 DISCUSSION OF LABORATORY TESTS PERFORMED
6.0 SITE CONDITIONS, GEOLOGIC SETTING, & TOPOGRAPHIC INFORMATION
7.0 SUMMARY OF SUBSURFACE CONDITIONS AND SOIL PROFILES
8.0 DISCUSSION OF FINDINGS, CONCLUSIONS, AND RECOMMENDATIONS
  8.1 GENERAL
    8.1.1 Subgrade & Foundation Soil/Rock Types
    8.1.2 Soil/Rock Properties
  8.2 GROUND WATER CONDITIONS/ OBSERVATIONS
  8.3 SPECIAL TOPICS (i.e., dynamic properties, seismicity, environmental).
  8.4 CHEMICAL ANALYSIS
9.0 FIELD PERMEABILITY TESTS
10.0 REFERENCES
LIST OF APPENDICES
  Appendix A - Boring Location Plan and Subsurface Profiles
  Appendix B - Test Boring Logs and Core Logs With Core Photographs
  Appendix C - Cone Penetration Test Soundings
  Appendix D - Flat Dilatometer, Pressuremeter, Vane Shear Test Results
  Appendix E - Geophysical Survey Data
  Appendix F - Field Permeability Test Data & Pumping Test Results
  Appendix G - Laboratory Test Results
  Appendix H - Existing Information
LIST OF FIGURES
LIST OF TABLES
GPR

Radar Waves

Object

Data Collection
Questions for Review

1. Compare the advantages and disadvantages between in-situ tests and drilling and lab testing.
2. Provide your recommendation of drilling/sampling, in-situ testing procedures for each of the following purposes, and state your assumptions for your answer if necessary:
   1. Determine the soil stratigraphy within the top 5 m, at 200 test locations along a proposed 20 km-long highway.
   2. Determine the compressibility of an overconsolidated clay layer at 5–15 m below ground surface.
   3. Determine the compressibility of a sand deposit at 5–15m below ground surface.
   4. Determine the groundwater table distribution within a 300-acre project site.
   5. Determine the type(s) and quality of rock from 50 to 100 m below ground surface.
3. Describe the possible procedures to determine the groundwater table in the field.
Assignment

Write a report including the following field tests:

1- Soil resistivity
2- Dynamic test for a soil, as megaphone
3- Pressure meter
4- Vane shear test
5- Ground penetration radar

End of this Lecture